

STORMWATER SOLUTIONS / STORAGE / CHAMBERS



Design Guide



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The HydroStor chamber represents the most advanced engineering, molding, and structural performance in the storm water management industry. Prinsco can assist in specifying HydroStor stormwater systems based on the project needs. Our expertise includes chamber layouts designed to meet volume requirements as well as inflow and outflow requirements for the HydroStor chamber system. Prinsco is also capable of assisting with conversions from other traditional stormwater management systems such as pipe, above ground ponds, vaults, and other manufactured and natural systems. The design engineer is responsible for determining that the layout meets all required design elements and is in compliance with the regulations which cover the project. For more information contact your local Prinsco representative at 1-800-992-1725.

1.0 Introduction

1.1 Introduction

The HydroStor chamber system was developed to provide an alternative storm water management system for design professionals, which provides a more efficient means of managing stormwater runoff from developed land. The applications for the HydroStor system include commercial, residential, industrial, highway, and agricultural drainage. HydroStor chambers deliver a lower cost solution by providing excellent stormwater management with the ability to tailor the solution to site requirements.

The HydroStor system has been designed and manufactured to exceed the requirements of the American Society for Testing and Materials (ASTM) standards. The injection molded structures provide the most cost effective, structurally superior chambers for the underground stormwater management market.

1.2 Product Design

HydroStor chambers are designed to meet the requirements of Section 12.12 of the American Association of State Highway and Transportation Officials (AASHTO) Load and Resistance Factor Design (LRFD) Bridge Design Specifications. The chambers are injection molded and manufactured to meet the requirements of ASTM F2418 *Standard Specification for Polypropylene (PP) Corrugated Wall Stormwater Collection Chambers*. Additionally, HydroStor chambers are designed in accordance with ASTM F2787 *Standard Practice for Structural Design of Thermoplastic Corrugated Wall Stormwater Collection Chambers*. The requirements of these ASTM and AASHTO standards dictate that the chambers follow rigorous design and quality requirements. All HydroStor chambers are manufactured at ISO 9001 compliant facilities.

1.3 Technical Support

During the design and construction of any underground stormwater management structure, it is frequently necessary to provide technical support for the product being designed and installed. Prinsco's team is committed to providing technical and engineering support for HydroStor chamber systems, including assistance with volume calculations, layout of the chamber system, and installation guidance. For technical assistance, contact your local Prinsco representative by calling 1-800-992-1725.

2.0 Product Information

2.1 Applications

HydroStor chambers were designed to primarily function as a stormwater detention and retention structure. In addition, they deliver additional benefits by providing both water quality and water quantity management. HydroStor chambers are an ideal storage system that maintains above ground uses such as parking lots, roadways, and green space.

HydroStor chambers provide the maximum flexibility for use on a construction site. Chamber systems can be configured in large beds or in individual trenches and can be used on almost any site to provide the required storage and improve water quality. The chamber systems can be retrofitted to sites that have already been developed and can be designed to efficiently and easily work around utilities, structures and other boundaries.

2.2 Stormwater Detention

Chamber systems have been used in stormwater detention for many years. A detention system is primarily used to hold stormwater before it is released from the site, reducing the peak flow rates of the site and restoring flow rates to that of pre-development conditions. The HydroStor stormwater chamber system is an open bottom structure that allows infiltration of stormwater into the ground. Most chamber systems are designed to provide a detention system type of control. There are limiting circumstances such as soft subgrades, karst topography, and expansive clay subgrades where infiltration may be undesirable. In situations such as these, a geomembrane liner can provide an excellent alternative to limit infiltration.

2.3 Chamber Sizing

The primary determining factors when choosing a chamber size are the depth to groundwater, bedrock, depth to other restrictive surface, available area for the underground system, cover height required over the system, and outlet invert restrictions.

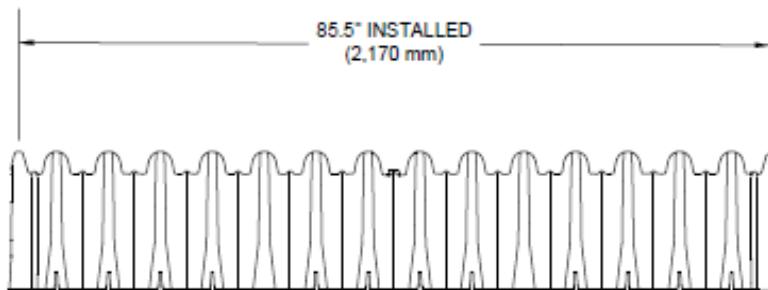
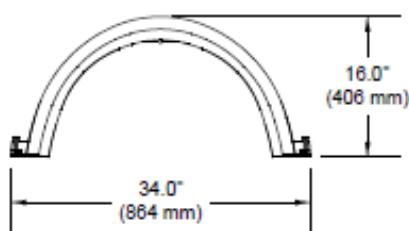
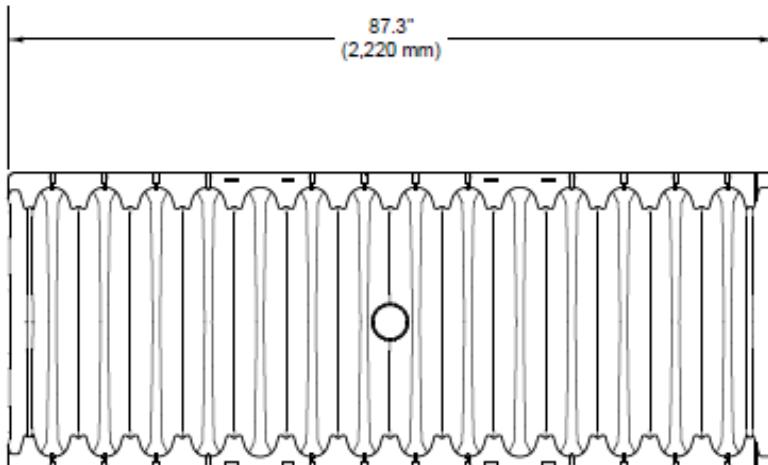
Because of differing site restrictions, HydroStor chambers are available in four sizes; two of which are detailed in this Design Guide due to similar installation properties. The HS31 has an installed capacity of 31 cubic feet (0.88 cubic meters) per chamber, based on a stone porosity of 40%. In many cases this is the optimal installation, especially for large footprints and limited vertical depth allowances for the installation.

For sites with slightly larger vertical room and a smaller horizontal footprint, the HS75 offers an excellent choice with an installed capacity of 75 cubic feet (2.12 cubic meters) chamber, based on a 40% stone porosity.

Illustrations of HydroStor HS31 and HS75 chambers and end caps are shown in Figures 1 and 2 below.



Chamber Specifications	
Chamber Size (L x W x H)	87.3" x 34.0" x 16.0" (2,220 x 864 x 406 mm)
Installed Length	85.5" (2,170 mm)
Chamber Storage	15.2 ft ³ (0.43 m ³)
Min. Installed Storage*	29.6 ft ³ (0.84 m ³)
Weight / Chamber	34 lbs (15 kg)
Chambers / Pallet	90
Approx. Weight / Pallet	3,380 lbs (1530 kg)



End Cap Specifications	
End Cap Size (L x W x H)	8.0" x 34.0" x 16.3" (200 x 864 x 414 mm)
Installed Length	6.9" (180 mm)
End Cap Storage	0.62 ft ³ (0.02 m ³)
Min. Installed Storage*	4.89 ft ³ (0.14 m ³)
Weight	3.8 lbs (1.7 kg)

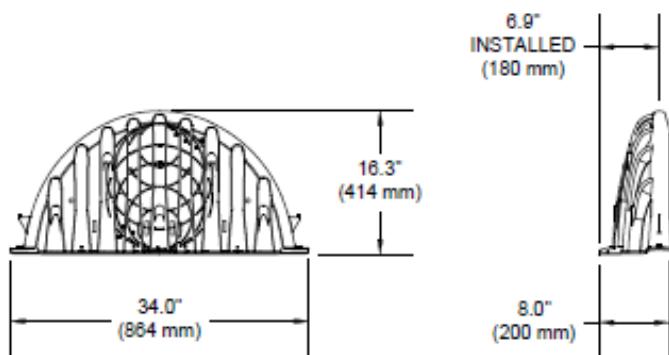
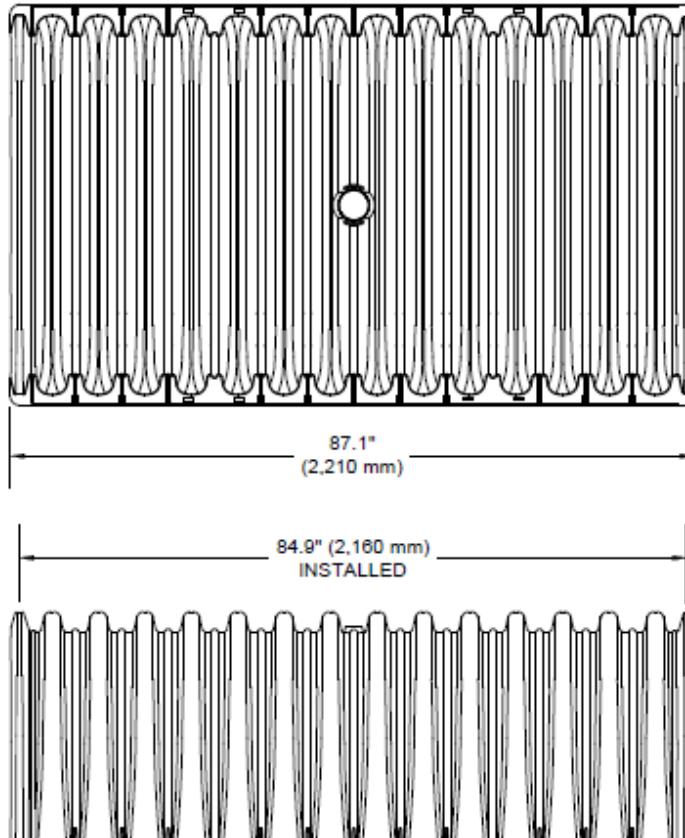
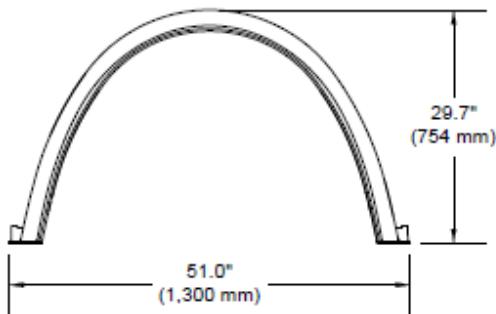


Figure 1 - HydroStor HS31 Chamber & End Caps



Chamber Specifications	
Chamber Size (L x W x H)	87.1" x 51.0" x 29.7" (2,210 x 1,300 x 754 mm)
Installed Length	84.9" (2,160 mm)
Chamber Storage	46.6 ft ³ (1.32 m ³)
Min. Installed Storage*	75.0 ft ³ (2.12m ³)
Weight / Chamber	69 lbs (31 kg)
Chambers / Pallet	33
Approx. Weight / Pallet	2,360 lbs (1,070 kg)



End Cap Specifications	
End Cap Size (L x W x H)	11.0" x 51.0" x 30.2" (302 x 1,300 x 767 mm)
Installed Length	10.4" (264 mm)
End Cap Storage	3.42 ft ³ (0.09 m ³)
Min. Installed Storage*	14.5 ft ³ (0.41 m ³)
Weight	13.2 lbs (5.99 kg)

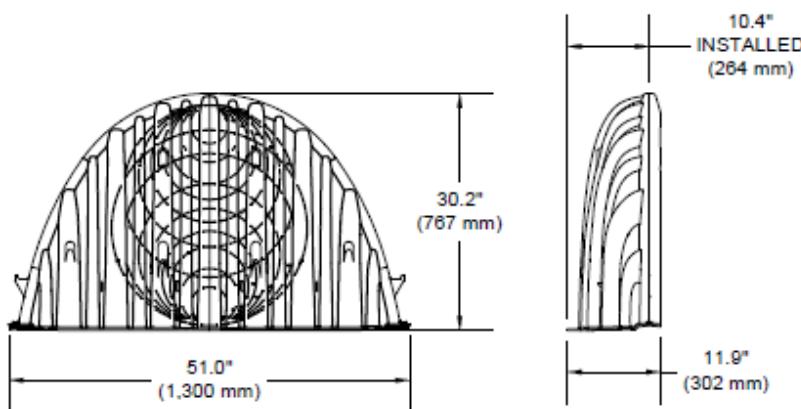


Figure 2 - HydroStor HS75 Chamber & End Caps

2.4 HydroStor Chambers

HydroStor Chambers are extremely versatile and can be configured as necessary to meet the site's needs. The chamber's versatility enables the chamber layout to be designed to appropriately meet the site conditions and avoid obstacles that may be interfering with a standard system layout. If a run of chambers must be interrupted, for instance, an end cap can be placed on the row's last chamber. The row can then be restarted after the interruption.

To ease construction and provide easy in-field guidance, each chamber is marked with overlap locations as well as the installation direction. (Figure 3) The assembly of each row is achieved by over topping the last rib of the initial chamber with the first rib of the succeeding chamber (Figure 4).

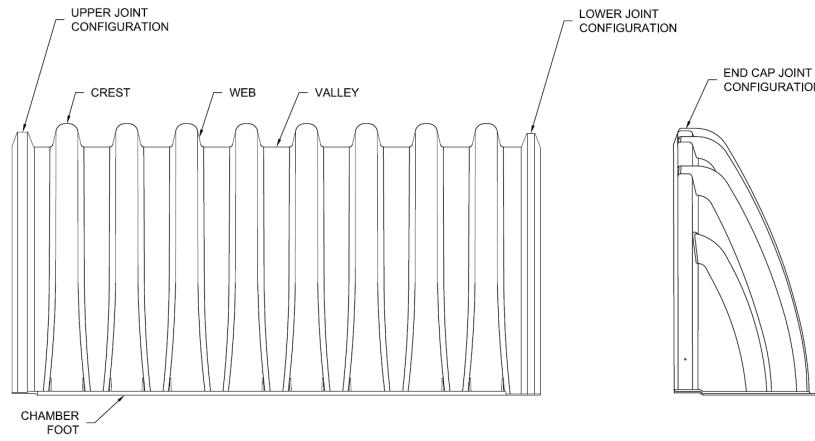


Figure 3 - Chamber and End Cap Components

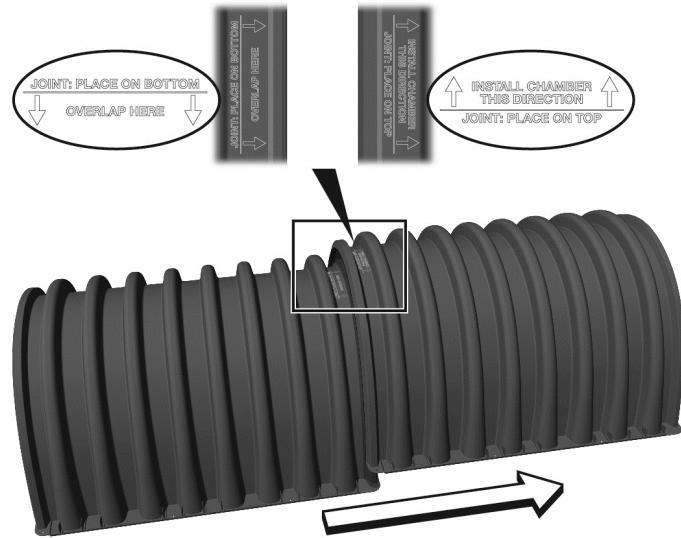


Figure 4 - Chamber Joint Assembly

2.5 End Caps

HydroStor end caps are designed to fit over either end of the chamber in order to terminate the chamber row. The end caps are required to be placed at the end of each row to seal the chamber from the aggregate surrounding it. This requires two end caps per row to be ordered with each chamber system. End caps for the HS31 will accept up to 10" (250 mm) pipe without an adapter and 12" (300 mm) pipe with an adapter for inlet and outlet to the row. End caps for the HS75 will accept up to 18" (450 mm) pipe without an adapter and 24" (600 mm) pipe with an adapter. Illustrations of HydroStor HS31 and HS75 end caps are shown in Figures 1 and 2. Figure 5 shows each end cap with the scribe lines to be used for field cuts, with respect to pipe diameter and pipe invert.

HS31 END CAP

TOP INVERTS

4" (100 mm) I.E. 0.86
6" (150 mm) I.E. 0.68
8" (200 mm) I.E. 0.46
10" (250 mm) I.E. 0.31

BOTTOM INVERTS
*12" (300 mm) I.E. 0.04
10" (250 mm) I.E. 0.11
8" (200 mm) I.E. 0.09
6" (150 mm) I.E. 0.08
4" (100 mm) I.E. 0.08
BOTTOM OF CHAMBER I.E. 0.00

*TO STUB IN A 12" PIPE ON THE HS31 ENDCAP, AN ADAPTER WILL HAVE TO BE USED.

HS75 END CAP

TOP INVERTS

4" (100 mm) I.E. 2.01
6" (150 mm) I.E. 1.83
8" (200 mm) I.E. 1.61
10" (250 mm) I.E. 1.46
12" (300 mm) I.E. 1.25
15" (375 mm) I.E. 0.99
18" (450 mm) I.E. 0.70

BOTTOM INVERTS
*24" (600 mm) I.E. 0.06
18" (450 mm) I.E. 0.20
15" (375 mm) I.E. 0.16
12" (300 mm) I.E. 0.16
10" (250 mm) I.E. 0.13
8" (200 mm) I.E. 0.11
6" (150 mm) I.E. 0.10
4" (100 mm) I.E. 0.09
BOTTOM OF CHAMBER I.E. 0.00

*TO STUB IN A 24" PIPE ON THE HS75 ENDCAP, AN ADAPTER WILL HAVE TO BE USED.

Figure 5 – End Cap Inverts

3.0 Structural Design

3.1 Design

Installation of HydroStor Chambers should be accomplished in accordance with the manufacturer's instructions. When installed correctly the HydroStor product line is designed to meet or exceed the American Association of State Highway and Transportation Officials (AASHTO) Load Resistance Factor Design (LRFD) design factors for dead loads (earth fill) and live loads (vehicular traffic). AASHTO vehicular live loads are designated as either a single axle 32 kip (142 kN) load or tandem 25 kip (111 kN) axle loads. The conservative design is based on additional factors for impact and multiple vehicles. The dead load applied is also subject to additional factors for safety.

Three dimensional models for the chambers were created. A Finite Element Analysis (FEA) was then utilized to determine required part thickness, weight, and shape to maximize the structural capacity of the product with an optimized profile. Maximum thrust, moment, local buckling, and deflection as well as other factors in both short term and long-term configurations were determined from the FEA.

The HydroStor chambers are designed for use in normal buried conditions and are not recommended for installation under structures, such as buildings, parking garages, or retaining walls as these could provide additional loading for which the chambers were not designed.

3.2 Testing

As part of the design and evaluation program, HydroStor chambers have been installed in field conditions to perform a variety of tests, including short term traffic loading, and continuous long-term testing and monitoring. The full-scale testing was designed to test the limits of the chambers and provide conclusive evidence that the product performs as designed and intended. Long term testing of HydroStor products is ongoing.

3.3 Molding

HydroStor chambers are constructed of polypropylene resin by injection molding. The entire system meets the design requirements of the AASHTO LRFD specifications as well as ASTM F2787 and ASTM F2418. The injection molding of the chambers provides a highly repeatable process for maintaining precise quality control of the parts as well as several other benefits including:

- Precise control of minimum wall thicknesses
- Precision fit for joints and end caps
- Engraved instructions for installation
- Handles on some Chamber sizes
- Uniform Material Placement
- Consistent Structural Strength

3.4 Quality Control

HydroStor chambers are manufactured with stringent quality control measures in place. The incoming raw materials are routinely tested to ensure compliance with the minimum requirements of the resin for processing and end product performance. The chamber properties are measured at industry standard intervals, ensuring proper performance when installed in accordance with the manufacturer's instructions.

4.0 Subsurface Foundation Requirements

4.1 Foundation Requirements

The soil on which the HydroStor system is constructed must have adequate bearing capacity to support the system. The native soils must be able to maintain the bearing capacity when water has infiltrated into the subsurface system as well. If there is inadequate bearing capacity of the native soil, additional aggregate may be necessary in the foundation of the system. The foundation must be clean, crushed, angular stone placed between the subgrade soil and the bottom of the chambers.

Increased dead load (fill) placed over top of the chamber system requires greater bearing capacity from the subgrade supporting the system. The strength of the foundation at the bottom of the chambers is determined by the bearing capacity of the subgrade soils and the depth of aggregate foundation stone that is placed. The deeper the stone placed in the foundation, the more the load is spread on the underlying subgrade allowing system installation in weaker native soils. Increased spacing between chambers allows either weaker subgrade soils or less foundation stone. Table 1a & 1b (for HS31 and HS75, respectively) provide guidance on foundation stone depth for varying cover heights and subgrade bearing capacity combinations based on the standard chamber spacing shown in Figure 6. These bearing capacity tables were calculated per ASTM F2787.

If soils with a bearing capacity of less than 2.0 ksf (96 kPa) or organic soils are encountered, a geotechnical engineer should evaluate the site conditions and make recommendations for installation of the system and required foundation preparation. The geotechnical engineer may recommend increasing the stone foundation or other remedial options. The site design engineer is responsible for determining acceptable settlement limits for the site.

Table 1a – HS31 Minimum Required Stone Foundation Depth, in (mm)

Cover Hgt. ft (m)	Minimum Required Bearing Capacity of Native Soil Subgrade, ksf (kPa)																					
	4.1 (196)	4.0 (192)	3.9 (187)	3.8 (182)	3.7 (177)	3.6 (172)	3.5 (168)	3.4 (163)	3.3 (158)	3.2 (153)	3.1 (148)	3.0 (144)	2.9 (139)	2.8 (134)	2.7 (129)	2.6 (124)	2.5 (120)	2.4 (115)	2.3 (110)	2.2 (105)	2.1 (101)	2.0 (96)
1.33 (0.41)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	12 (305)	12 (305)	12 (305)	12 (305)	15 (381)	15 (381)							
2 (0.61)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)	12 (305)	12 (305)	12 (305)	15 (381)	15 (381)	
2.5 (0.76)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	12 (305)	12 (305)	
3 (0.91)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)	12 (305)
3.5 (1.07)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)
4 (1.22)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)	12 (305)
4.5 (1.37)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)
5 (1.52)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)
5.5 (1.68)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)
6 (1.83)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)
6.5 (1.98)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)
7 (2.13)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)
7.5 (2.29)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)
8 (2.44)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	15 (381)

Note: The minimum foundation stone thickness is to be determined by the site design engineer based on the bearing capacity of the subgrade soil with consideration for range of expected soil moisture conditions.

Table 2b – HS75 Minimum Required Stone Foundation Depth, in (mm)

Cover Height ft (m)	Minimum Required Bearing Capacity of Native Soil Subgrade, ksf (kPa)																				
	4.1 (196)	4.0 (192)	3.9 (187)	3.8 (182)	3.7 (177)	3.6 (172)	3.5 (168)	3.4 (163)	3.3 (158)	3.2 (153)	3.1 (148)	3.0 (144)	2.9 (139)	2.8 (134)	2.7 (129)	2.6 (124)	2.5 (120)	2.4 (115)	2.3 (110)	2.2 (105)	2.1 (101)
1.5 (0.46)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)	12 (305)	12 (305)	15 (381)
2 (0.61)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)	12 (305)	12 (305)	15 (381)
2.5 (0.76)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)	12 (305)	12 (305)	15 (381)
3 (0.91)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)	12 (305)	12 (305)	15 (381)
3.5 (1.07)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)	12 (305)	12 (305)	15 (381)
4 (1.22)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)	12 (305)	12 (305)	15 (381)
4.5 (1.37)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)	12 (305)	12 (305)	15 (381)
5 (1.52)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)	12 (305)	12 (305)	15 (381)
5.5 (1.68)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)	12 (305)	12 (305)	15 (381)
6 (1.83)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)	12 (305)	12 (305)	15 (381)
6.5 (1.98)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)	12 (305)	12 (305)	15 (381)
7 (2.13)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)	12 (305)	12 (305)	15 (381)
7.5 (2.29)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	6 (152)	9 (229)	9 (229)	9 (229)	9 (229)	9 (229)	12 (305)	12 (305)	12 (305)	15 (381)
8 (2.44)	6 (152)	6 (152)	6 (229)	9 (305)	9 (305)	9 (305)	9 (305)	9 (305)	15 (381)	15 (381)	15 (381)	18 (457)									

Note: The minimum foundation stone thickness is to be determined by the site design engineer based on the bearing capacity of the subgrade soil with consideration for range of expected soil moisture conditions.

4.2 Chamber Row Spacing

The minimum required spacing between chamber rows is 6 inches (150 mm) for HS75 and 3 inches (75 mm) for the HS31. This spacing is measured from the outside edges of the chamber feet. The spacing may be increased, if necessary, to mitigate the effects of weaker native subgrade soils or to reduce the amount of foundation stone required under the chambers.

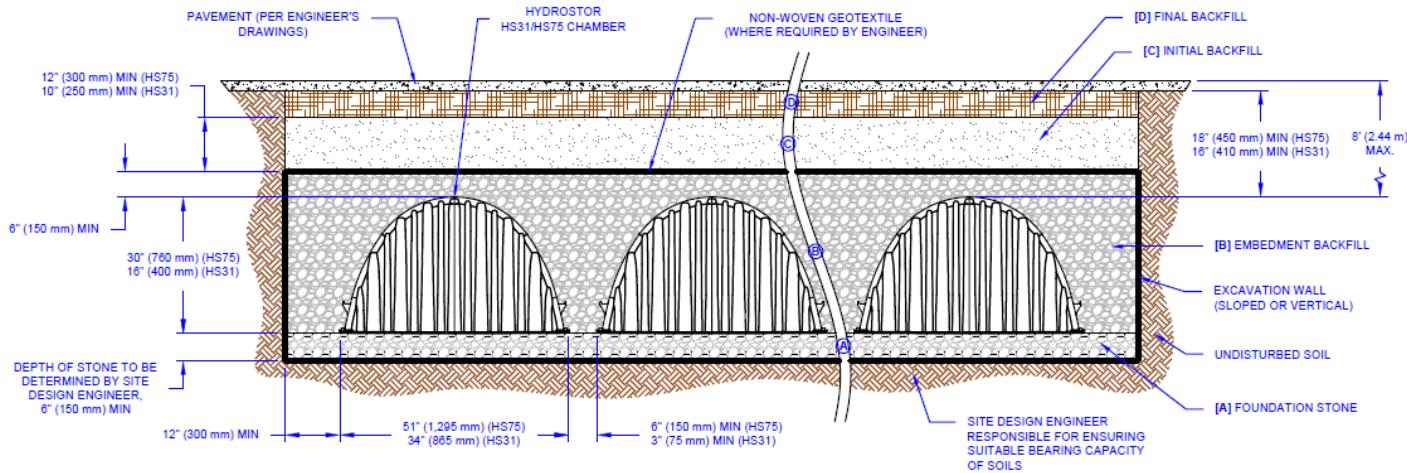


Figure 6 - HydroStor Chamber Cross Section

5.0 Installed Storage Volumes for HydroStor Chambers

5.1 Stage Storage Volumes

Tables 2 and 3 below provide the cumulative storage volumes for the HydroStor HS75 chamber and HS31 chambers, respectively. These tables can be used to calculate the stage-storage volumes for the retention or detention systems. If a thicker foundation layer is required, additional stone void storage would be possible. The storage in the installed system assumes a 40% stone porosity.

Table 2 – HS31 Incremental Storage Volume per Chamber

Calculations assume 6" (150 mm) of stone above chambers, 6" stone for foundation, 3" (75 mm) of stone between chambers and 40% stone porosity. Add 0.73 ft³ (0.02 m³) for each additional inch of stone foundation or cover.

Depth of water in the System in. (mm)	Cumulative Single Chamber Storage ft ³ (m ³)	Cumulative Storage for System ft ³ (m ³)	Depth of water in the System in. (mm)	Cumulative Single Chamber Storage ft ³ (m ³)	Cumulative Storage for System ft ³ (m ³)
28 (711)	↑ 15.14 (0.43)	29.56 (0.84)	14 (356)	10.19 (0.29)	16.36 (0.46)
27 (686)	15.14 (0.43)	28.83 (0.82)	13 (330)	9.05 (0.26)	14.94 (0.42)
26 (660)	15.14 (0.43)	28.1 (0.8)	12 (305)	7.86 (0.22)	13.49 (0.38)
25 (635)	Stone Cover 15.14 (0.43)	27.37 (0.78)	11 (279)	6.63 (0.19)	12.02 (0.34)
24 (610)	15.14 (0.43)	26.64 (0.75)	10 (254)	5.37 (0.15)	10.53 (0.3)
23 (584)	↓ 15.14 (0.43)	25.91 (0.73)	9 (229)	4.07 (0.12)	9.02 (0.26)
22 (559)	15.14 (0.43)	25.18 (0.71)	8 (203)	2.74 (0.08)	7.49 (0.21)
21 (533)	15.09 (0.43)	24.42 (0.69)	7 (178)	1.38 (0.04)	5.94 (0.17)
20 (508)	14.89 (0.42)	23.57 (0.67)	6 (152)	↑ 0 (0)	4.38 (0.12)
19 (483)	14.55 (0.41)	22.63 (0.64)	5 (127)	0 (0)	3.65 (0.1)
18 (457)	13.94 (0.39)	21.53 (0.61)	4 (102)	Stone 0 (0)	2.92 (0.08)
17 (432)	13.16 (0.37)	20.33 (0.58)	3 (76)	Foundation 0 (0)	2.19 (0.06)
16 (406)	12.26 (0.35)	19.06 (0.54)	2 (51)	0 (0)	1.46 (0.04)
15 (381)	11.26 (0.32)	17.73 (0.5)	1 (25)	↓ 0 (0)	0.73 (0.02)

Table 3 - HS75 Incremental Storage Volume per Chamber

Calculations assume 6" (150 mm) of stone above chambers, 6" stone for foundation, 6" (150 mm) of stone between chambers and 40% stone porosity. Add 1.12 ft³ (0.032 m³) for each additional inch of stone foundation or cover.

Depth of water in the System in. (mm)	Cumulative Single Chamber Storage ft ³ (m ³)	Cumulative Storage for System ft ³ (m ³)	Depth of water in the System in. (mm)	Cumulative Single Chamber Storage ft ³ (m ³)	Cumulative Storage for System ft ³ (m ³)
42 (1067)	46.6 (1.32)	74.99 (2.12)	21 (533)	30.38 (0.86)	41.75 (1.18)
41 (1041)	46.6 (1.32)	73.87 (2.09)	20 (508)	28.61 (0.81)	39.57 (1.12)
40 (1016)	Stone 46.6 (1.32)	72.75 (2.06)	19 (483)	26.79 (0.76)	37.36 (1.06)
39 (991)	Cover 46.6 (1.32)	71.63 (2.03)	18 (457)	24.93 (0.71)	35.12 (0.99)
38 (965)	46.6 (1.32)	70.51 (2)	17 (432)	23.03 (0.65)	32.86 (0.93)
37 (940)	↓ 46.6 (1.32)	69.39 (1.96)	16 (406)	21.09 (0.6)	30.58 (0.87)
36 (914)	46.6 (1.32)	68.27 (1.93)	15 (381)	19.12 (0.54)	28.28 (0.8)
35 (889)	46.56 (1.32)	67.13 (1.9)	14 (356)	17.11 (0.48)	25.95 (0.73)
34 (864)	46.38 (1.31)	65.9 (1.87)	13 (330)	15.07 (0.43)	23.61 (0.67)
33 (838)	46.08 (1.3)	64.6 (1.83)	12 (305)	13 (0.37)	21.25 (0.6)
32 (813)	45.45 (1.29)	63.1 (1.79)	11 (279)	10.9 (0.31)	18.87 (0.53)
31 (787)	44.59 (1.26)	61.46 (1.74)	10 (254)	8.77 (0.25)	16.47 (0.47)
30 (762)	43.57 (1.23)	59.73 (1.69)	9 (229)	6.62 (0.19)	14.06 (0.4)
29 (737)	42.43 (1.2)	57.93 (1.64)	8 (203)	4.44 (0.13)	11.63 (0.33)
28 (711)	41.18 (1.17)	56.06 (1.59)	7 (178)	2.23 (0.06)	9.18 (0.26)
27 (686)	39.84 (1.13)	54.14 (1.53)	6 (152)	0 (0)	6.72 (0.19)
26 (660)	38.42 (1.09)	52.17 (1.48)	5 (127)	0 (0)	5.6 (0.16)
25 (635)	36.93 (1.05)	50.16 (1.42)	4 (102)	Stone 0 (0)	4.48 (0.13)
24 (610)	35.38 (1)	48.11 (1.36)	3 (76)	Foundation 0 (0)	3.36 (0.1)
23 (584)	33.77 (0.96)	46.02 (1.3)	2 (51)	0 (0)	2.24 (0.06)
22 (559)	32.1 (0.91)	43.9 (1.24)	1 (25)	↓ 0 (0)	1.12 (0.03)

Table 4 – HS31 Incremental Storage Volume per End Cap

Calculations assume 6" (150 mm) of stone above chambers, 6" stone for foundation, 3" (75 mm) of stone between chambers and 40% stone porosity. Add 0.16 ft³ (0.005 m³) for each additional inch of stone foundation or cover.

Depth of water in the System in. (mm)	Cumulative Single End Cap Storage ft ³ (m ³)	Cumulative Storage for System ft ³ (m ³)	Depth of water in the System in. (mm)	Cumulative Single End Cap Storage ft ³ (m ³)	Cumulative Storage for System ft ³ (m ³)
28 (711)	0.62 (0.02)	4.89 (0.14)	14 (356)	0.43 (0.01)	2.52 (0.07)
27 (686)	0.62 (0.02)	4.73 (0.13)	13 (330)	0.39 (0.01)	2.33 (0.07)
26 (660)	Stone 0.62 (0.02)	4.57 (0.13)	12 (305)	0.34 (0.01)	2.14 (0.06)
25 (635)	Cover 0.62 (0.02)	4.41 (0.12)	11 (279)	0.29 (0.01)	1.95 (0.06)
24 (610)	0.62 (0.02)	4.25 (0.12)	10 (254)	0.24 (0.01)	1.76 (0.05)
23 (584)	0.62 (0.02)	4.09 (0.12)	9 (229)	0.18 (0.01)	1.56 (0.04)
22 (559)	0.62 (0.02)	3.93 (0.11)	8 (203)	0.12 (0)	1.36 (0.04)
21 (533)	0.62 (0.02)	3.77 (0.11)	7 (178)	0.06 (0)	1.16 (0.03)
20 (508)	0.61 (0.02)	3.6 (0.1)	6 (152)	0 (0)	0.96 (0.03)
19 (483)	0.59 (0.02)	3.43 (0.1)	5 (127)	0 (0)	0.8 (0.02)
18 (457)	0.57 (0.02)	3.26 (0.09)	4 (102)	Stone 0 (0)	0.64 (0.02)
17 (432)	0.54 (0.02)	3.08 (0.09)	3 (76)	Foundation 0 (0)	0.48 (0.01)
16 (406)	0.51 (0.01)	2.9 (0.08)	2 (51)	0 (0)	0.32 (0.01)
15 (381)	0.47 (0.01)	2.71 (0.08)	1 (25)	0 (0)	0.16 (0)

Table 5 – HS75 Incremental Storage Volume per End Cap

Calculations assume 6" (150 mm) of stone above chambers, 6" stone for foundation, 6" (150 mm) of stone between chambers and 40% stone porosity. Add 0.30 ft³ (0.008 m³) for each additional inch of stone foundation or cover.

Depth of water in the System in. (mm)	Cumulative Single End Cap Storage ft ³ (m ³)	Cumulative Storage for System ft ³ (m ³)	Depth of water in the System in. (mm)	Cumulative Single End Cap Storage ft ³ (m ³)	Cumulative Storage for System ft ³ (m ³)
42 (1067)	3.4 (0.1)	14.53 (0.41)	21 (533)	2.29 (0.06)	7.62 (0.22)
41 (1041)	3.4 (0.1)	14.23 (0.4)	20 (508)	2.16 (0.06)	7.25 (0.21)
40 (1016)	Stone 3.4 (0.1)	13.93 (0.39)	19 (483)	2.02 (0.06)	6.87 (0.19)
39 (991)	Cover 3.4 (0.1)	13.63 (0.39)	18 (457)	1.88 (0.05)	6.49 (0.18)
38 (965)	3.4 (0.1)	13.33 (0.38)	17 (432)	1.74 (0.05)	6.11 (0.17)
37 (940)	3.4 (0.1)	13.03 (0.37)	16 (406)	1.59 (0.05)	5.72 (0.16)
36 (914)	3.4 (0.1)	12.73 (0.36)	15 (381)	1.44 (0.04)	5.33 (0.15)
35 (889)	3.4 (0.1)	12.43 (0.35)	14 (356)	1.29 (0.04)	4.94 (0.14)
34 (864)	3.39 (0.1)	12.13 (0.34)	13 (330)	1.13 (0.03)	4.55 (0.13)
33 (838)	3.36 (0.1)	11.82 (0.33)	12 (305)	0.97 (0.03)	4.16 (0.12)
32 (813)	3.32 (0.09)	11.5 (0.33)	11 (279)	0.81 (0.02)	3.77 (0.11)
31 (787)	3.27 (0.09)	11.17 (0.32)	10 (254)	0.65 (0.02)	3.38 (0.1)
30 (762)	3.21 (0.09)	10.84 (0.31)	9 (229)	0.49 (0.01)	2.99 (0.08)
29 (737)	3.14 (0.09)	10.5 (0.3)	8 (203)	0.32 (0.01)	2.59 (0.07)
28 (711)	3.06 (0.09)	10.16 (0.29)	7 (178)	0.15 (0)	2.19 (0.06)
27 (686)	2.97 (0.08)	9.81 (0.28)	6 (152)	0 (0)	1.8 (0.05)
26 (660)	2.87 (0.08)	9.45 (0.27)	5 (127)	0 (0)	1.5 (0.04)
25 (635)	2.77 (0.08)	9.09 (0.26)	4 (102)	Stone 0 (0)	1.2 (0.03)
24 (610)	2.66 (0.08)	8.73 (0.25)	3 (76)	Foundation 0 (0)	0.9 (0.03)
23 (584)	2.54 (0.07)	8.36 (0.24)	2 (51)	0 (0)	0.6 (0.02)
22 (559)	2.42 (0.07)	7.99 (0.23)	1 (25)	0 (0)	0.3 (0.01)

6.0 Required Materials for Installation

6.1 Chamber Row Separation

The HS31 and HS75 chambers are designed to have a minimum of 3 inches (75 mm) and 6 inches (150 mm), respectively, of spacing between each chamber with the footing of each chamber being parallel to the one next to it (Figure 7). Wider spacing is acceptable and will increase the storage with the additional stone voids as well as spreading the load on the native subgrade material. A brick or other spacer is recommended to maintain the required spacing between the chamber feet.

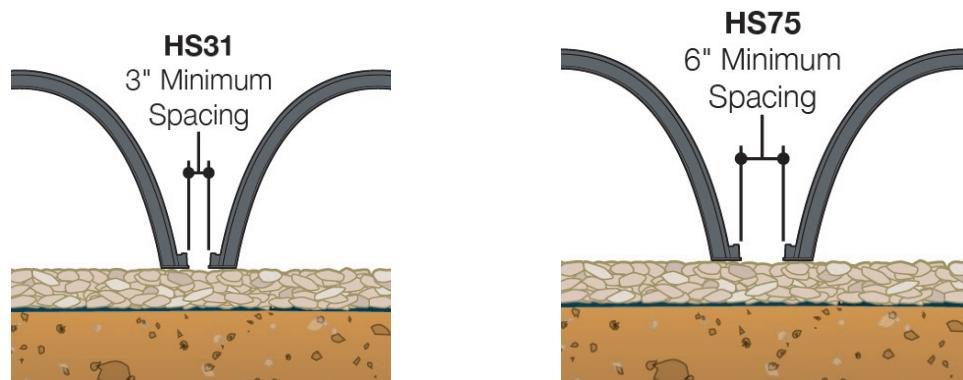


Figure 7 - Minimum Chamber Row Spacing

6.2 Stone Backfill

The acceptable stone materials for each of the backfill layers are shown in Table 6, along with backfill placement and compaction recommendations. The embedment stone which is used under, in between, and above the chambers must be clean, crushed, and angular. Figure 6 depicts a typical installation cross section for the HS75 and HS31 chambers. Table 6 and Figure 6 indicate the appropriate locations and types of backfill required for the installation of the chamber system. The material above the clean, crushed, angular backfill used to envelope the chambers (layer D) can be as specified by the engineer to provide adequate subbase for the overlying structures. Minimum and maximum fill requirements are also shown in Figure 6. The minimum cover for non-paved installations is 24" (600 mm) for HS75 and 22" (560 mm) for HS31 chambers.

Table 6 - Acceptable Backfill Requirements

Fill Material Location	Material Description	AASHTO M43 Designation	HS31 and HS75 Compaction Requirements
[D] Final Backfill- Fill material for Layer D starts at the top of the C layer to the bottom of the flexible pavement or to the finished grade of an unpaved surface. The pavement subbase may be part of the final backfill.	Any backfill which provides adequate subgrade for the project per the engineer's plans. Plans shall indicate subgrade requirements.	N/A	Prepared per site design engineer's plans.
[C] Initial Backfill- Material for layer C starts at the top of the embedment zone (layer B) and continues to 18" (450 mm) above the top of chamber. The pavement subbase may be part of the initial backfill layer.	Well graded granular material, <35% fines. Most pavement subbase materials can be used in lieu of this layer.	AASTHO M45 A-1, A-2, A-3 Or AASTHO M43 3, 357, 4, 467, 5, 56, 57, 6, 67, 68, 7, 78, 8, 89, 9, 10	Compaction will not begin until a minimum of 12" (300 mm) of material is placed over the chambers. Additional layers shall be compacted in 6" (150 mm) lifts to a minimum of 95% standard proctor density for well graded material. Roller gross vehicles are not to exceed 12,000 lbs (53 kN), and dynamic force not to exceed 20,000 lbf (89 kN).
[B] Embedment Stone- Embedment stone will surround the chambers and extends from the top of the foundation stone (layer A) to the bottom of the fabric layer.	3/4" (19 mm) to 2" (51 mm) clean, crushed, angular stone.	AASHTO M43 3, 357, 4, 467, 5, 56, 57	No compaction required
[A] Foundation Stone- Foundation Stone extends from the sub grade to the foot of the chambers.	3/4" (19 mm) to 2" (51 mm) clean, crushed, angular stone.	AASHTO M43 3, 357, 4, 467, 5, 56, 57	Placed in 6" (150 mm) lifts and compacted with a vibratory compactor.

6.3 Geotextile

The stone used in the foundation and backfill around the chambers is required to be open graded for water storage in the voids. Therefore, it is necessary to protect the system from infiltration of the surrounding native soil. A non-woven geotextile is to be used around the entire chamber system to prevent soil infiltration into the crushed, angular stone around the chambers. The non-woven geotextile shall be AASHTO M288 Class 2 or Class 3 non-woven geotextile. The geotextile shall be overlapped a minimum of 24" (600 mm) as shown in Figure 8.

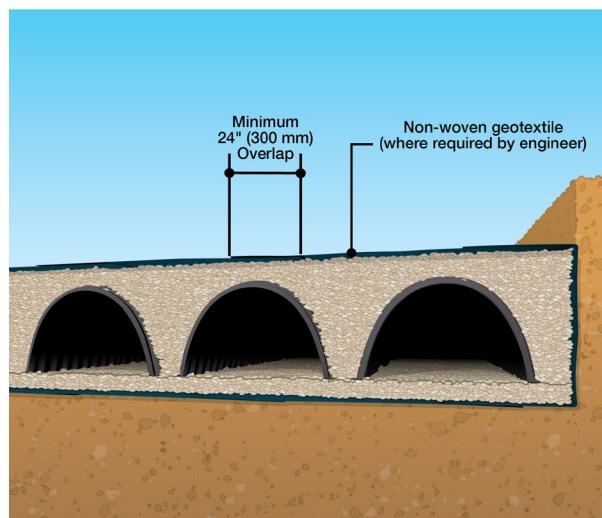


Figure 8 – AASHTO M288 Class 2 or Class 3 Separation Fabric

7.0 Stormwater Flow into Chambers

There are numerous ways of routing the stormwater from a developed site into the HydroStor chambers. Considering chambers have open bottoms, it is essential to utilize a pretreatment system to remove sediment and debris before the influent enters the chamber system. Below is some guidance on alternatives for the treatment of the inlet flow. Should more assistance be required, please contact your local Prinsco representative.

7.1 Retention/Detention and the Treatment Train

In addition to the retention / detention of stormwater, one of the functions of an underground system is the treatment of stormwater for water quality. When properly designed the HydroStor system can be combined with other stormwater treatment devices to provide a multi-tiered treatment approach for water quality, creating a very effective treatment train. The treatment train can consist of up to three levels of treatment for stormwater entering the HydroStor system including the following:

- Pre-Treatment Systems
- HydroStor Sediment Row
- Enhanced Stormwater Treatment

7.2 Pre-Treatment Systems

Depending on the location of the project, pre-treatment may be required prior to the stormwater entering the system. A pretreatment device can be used to reduce the pollutant load entering the system. By reducing the load entering the system, the service life of the system can be extended and sediment as well as other pollutants can be captured helping to meet local regulations.

The types of pretreatment systems can differ in method and effectiveness. A pretreatment device such as Prinsco's Stormwater Quality Unit (SWQU) treats the initial stormwater runoff before it reaches the chamber system. (Figure 9) Several standard units are available. Custom units, based on job specific requirements, can be manufactured. Refer to Prinsco's SWQU Product Detail & Bypass detail or contact your local Prinsco representative for more information.

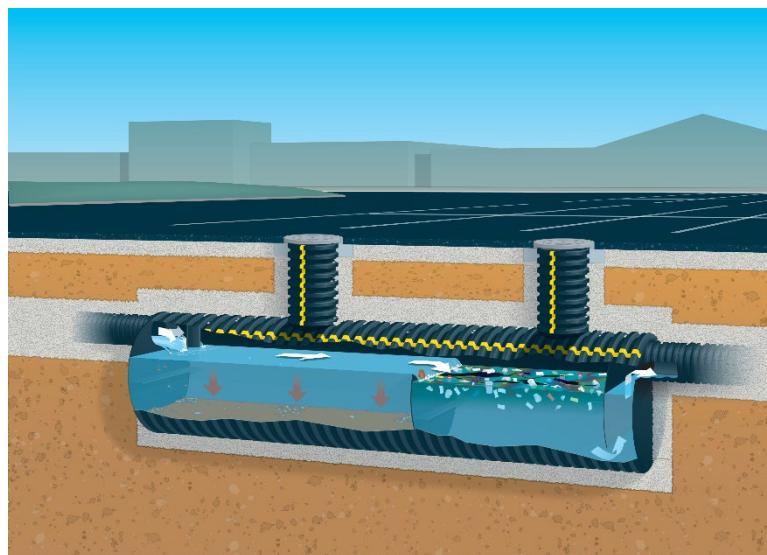


Figure 9 - Prinsco Stormwater Quality Unit (SWQU)

Other simple systems can potentially provide the level of treatment required based on the site's characteristics and the required treatment standards of the local regulations. Other treatment options include a manhole with a sump, gravity separators, hydrodynamic separators, and low impact development methods such as swales and grass strips.

The design engineer on the project is responsible for designing the appropriate water quality system based on the local regulations and requirements.

7.3 HydroStor Sediment Row

The HydroStor sediment row provides additional treatment for stormwater quality and enhances the removal of Total Suspended Solids. The sediment row consists of a series of chambers installed directly on top of two layers of woven geotextile fabric and is connected to a diversion manhole. The two layers of nonwoven geotextile directly beneath the chambers provide filtration of water leaving the row, capturing sediment and other pollutants within the row. Water in the sediment row has no outlet other than the bottom of the chamber and is forced through the filter fabric, providing the necessary filtration.

The sediment row is designed with a manhole and an elevated bypass manifold at the upstream end. The elevated manifold allows the initial runoff at the beginning of a storm, often referred to as the “first flush”, to enter the sediment row for filtration. Once the bypass height is reached, the remaining water flows through the manifold header and is distributed to the additional rows of the HydroStor chamber system. The manhole is connected to the sediment row with a short length of HDPE pipe up to 18" (450 mm) for the HS75 and 10" (250 mm) for the HS31. Note a 24" (600 mm) connection to HS75 systems and a 12" (300 mm) connection to HS31 is possible with adapter. The downstream end of the sediment row has an end cap with no outlet.

Two layers of a woven geotextile, meeting the requirements of AASHTO M288 Class 1 stabilization fabric, are placed between bottom of the chambers and the stone foundation. This strong fabric allows water to pass through the chamber into the base stone yet keeps sediment and other pollutants within the sediment row.

In addition the fabric provides a strong base for cleaning the system using a JetVac type of technology. It is recommended that the two geotextile layers do not include any joints, overlaps or seams.

Inspection of the sediment row can be accomplished through the diversion manhole or through inspection ports which can be installed on the chambers. The maintenance of the system and cleanout is accomplished through the use of a JetVac process. The amount of sediment retained and the amount of water which is treated is based on the size of the project and the age of the system. Contact your local Prinsco representative for assistance on sizing of the sediment row. Further detail on the sediment row is shown in Figure 10.

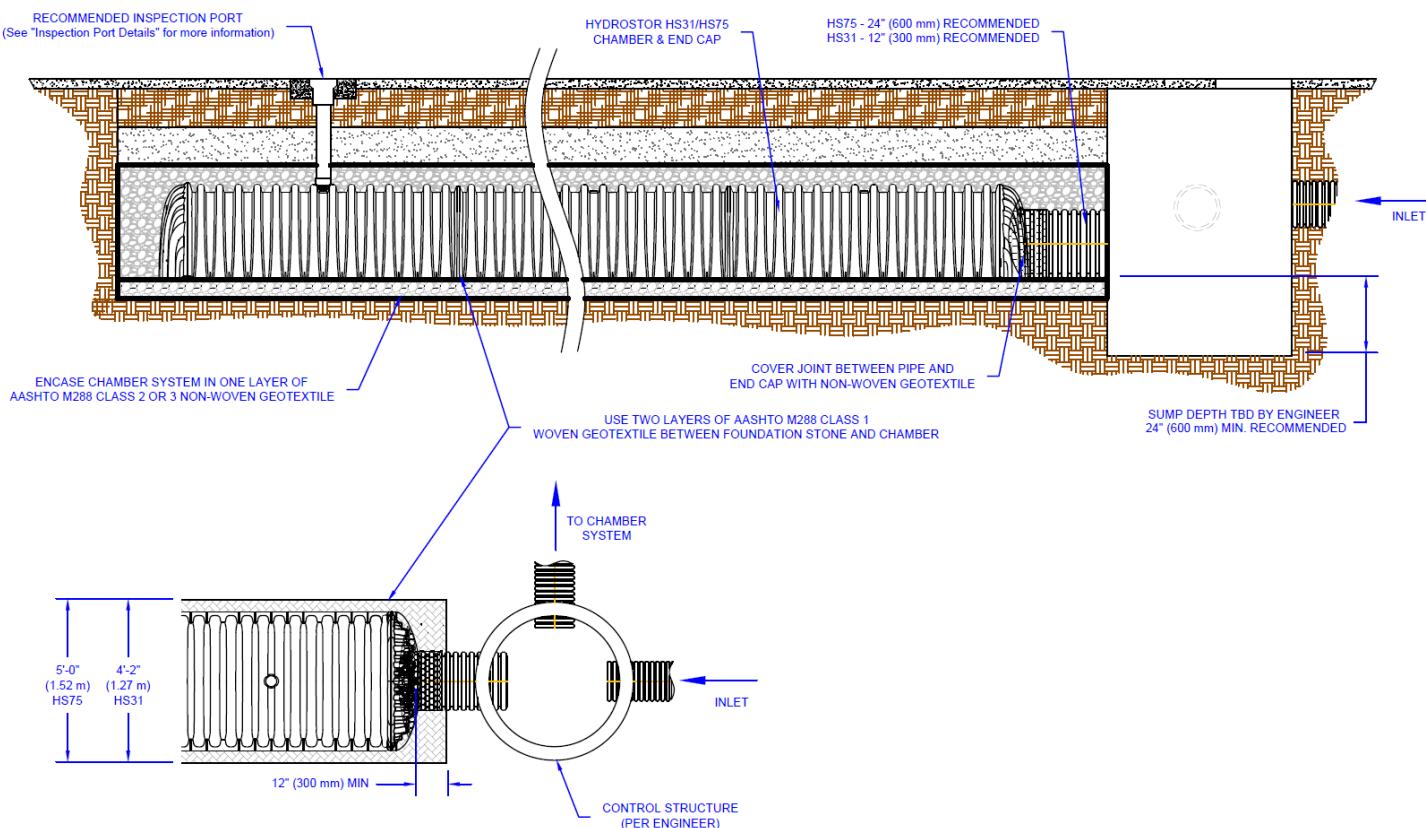


Figure 10 - Sediment Row

7.4 Enhanced Stormwater Treatment

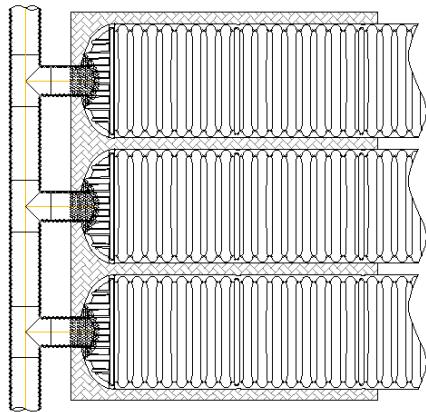
In some regions, regulations may require further sediment removal beyond what a standard pretreatment device and sediment row are capable of. In these cases, the HydroStor system can easily be designed to accommodate further stormwater treatment either before or after the HydroStor chamber system. The location of the enhanced treatment is dependent on local regulations, the requirements of the site, and design engineer preferences. Contact your local Prinsco representative for further recommendations on enhanced treatment.

7.5 Treatment Train

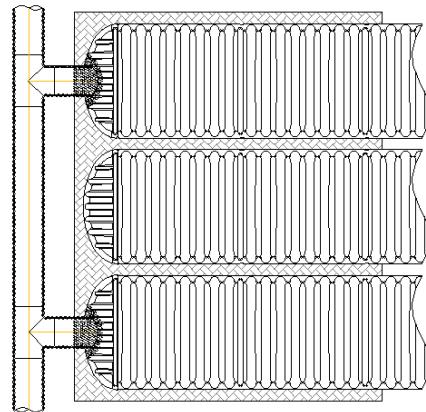
The combination of all treatment devices is commonly referred to as the “treatment train”. Each device is intended to treat the stormwater to a higher level, removing smaller pollutants as the stormwater progresses through the system. The HydroStor chamber system provides the ability to not only store and treat water, but also work in combination with other devices to provide a comprehensive water quality solution.

7.6 Additional Inlet Options

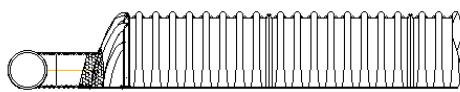
Treatment of the stormwater is recommended prior to inletting into the chamber system. Depending on the type of pretreatment system, a diversion manhole may not be necessary and only the header system would be used to inlet the stormwater into the chamber bed. For inlet options using this alternative, see Figure 11. Contact your local Prinsco representative for a unique and cost-effective solution for your stormwater project.



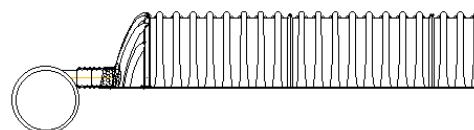
INLET MANIFOLD WITH STUBS INTO END CAP OF EACH STORM CHAMBER ROW.



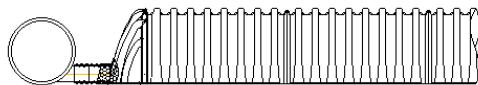
INLET MANIFOLD WITH STUBS INTO END CAP OF ALTERNATING STORM CHAMBER ROWS.



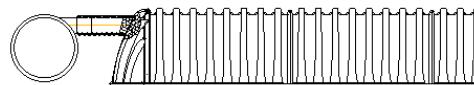
HEADER WITH SAME SIZE MANIFOLD STUB INTO END CAP.



HEADER WITH REDUCING STUB AT CROWN INTO INVERT OF END CAP WITH HEADER INVERT BELOW CHAMBER INVERT.



HEADER WITH REDUCING STUB AT FLOWLINE INTO END CAP.



HEADER WITH REDUCING STUB AT CROWN INTO TOP OF END CAP WITH HEADER INVERT AT OR BELOW STUB INVERT.

Figure 11 - Inlet Manifold Options

7.7 Perpendicular Inlets

In most installations, inlets to the chambers will be at the chamber row ends with a header system. However, an inlet perpendicular to the system may be desirable in some cases. If this is required, a chamber can be removed from a row and replaced with a tee connected to the inlet pipe. (Figure 12) The tee is then connected to the end caps of the chambers adjacent to the tee. A connection directly into the side of a chamber wall is not recommended.

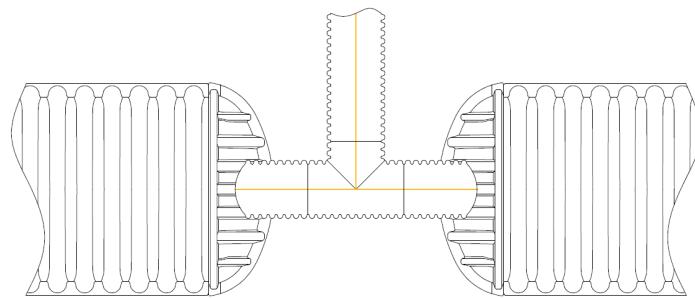


Figure 12 – Perpendicular Chamber Connection

7.8 Maximum Inlet Velocity

Water entering a chamber row too rapidly can potentially scour the foundation stone, leading to foundation issues in the chamber system. Prinsco recommends using a minimum 12.5' (3.81 m) wide strip of AASHTO M288 Class 1 woven stabilization geotextile beneath each chamber inlet row to protect the stone from scour and structural loss (Figure 13).

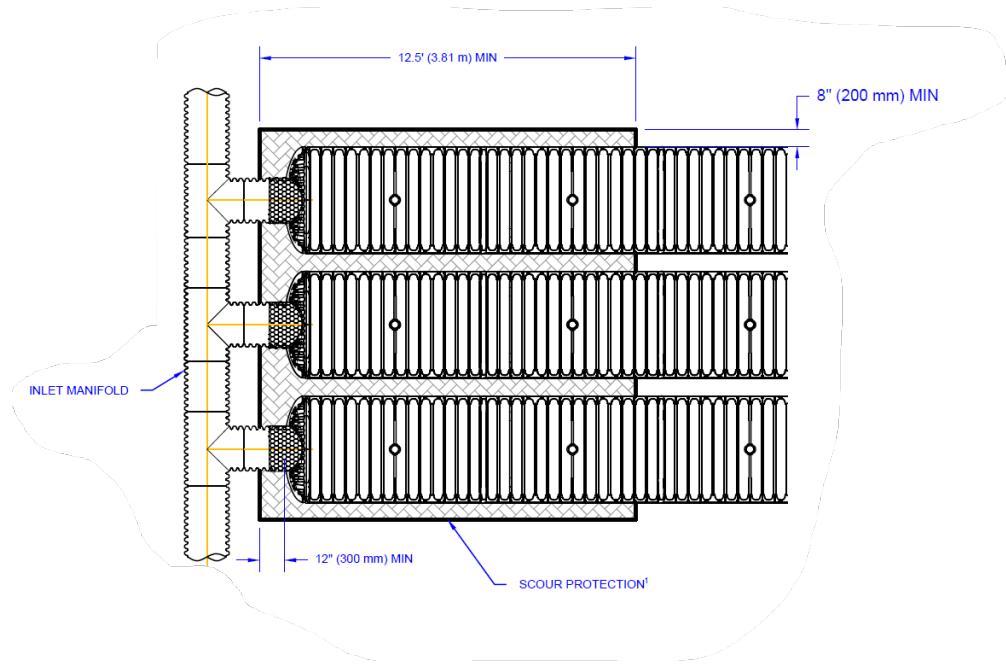


Figure 13 - Inlet Scour Protection

8.0 Outlet from HydroStor Chamber Systems

Most installations of chamber systems are used for detention and will therefore require some type of outlet structure. In detention design, the outlet structure is usually designed to match the pre-developed flow rates for each storm as closely as possible.

Since the chamber system utilizes the stone surrounding it for additional storage, an underdrain system may be necessary to completely drain the system. The underdrain must be located in or under the foundation stone, preferably in a sump area to completely drain the system. If a sump is used, it is usually unnecessary to slope the base to provide drainage. The underdrain should not be installed under the chambers. Figure 14 depicts an optional underdrain outlet configuration with minimum recommended dimensions.

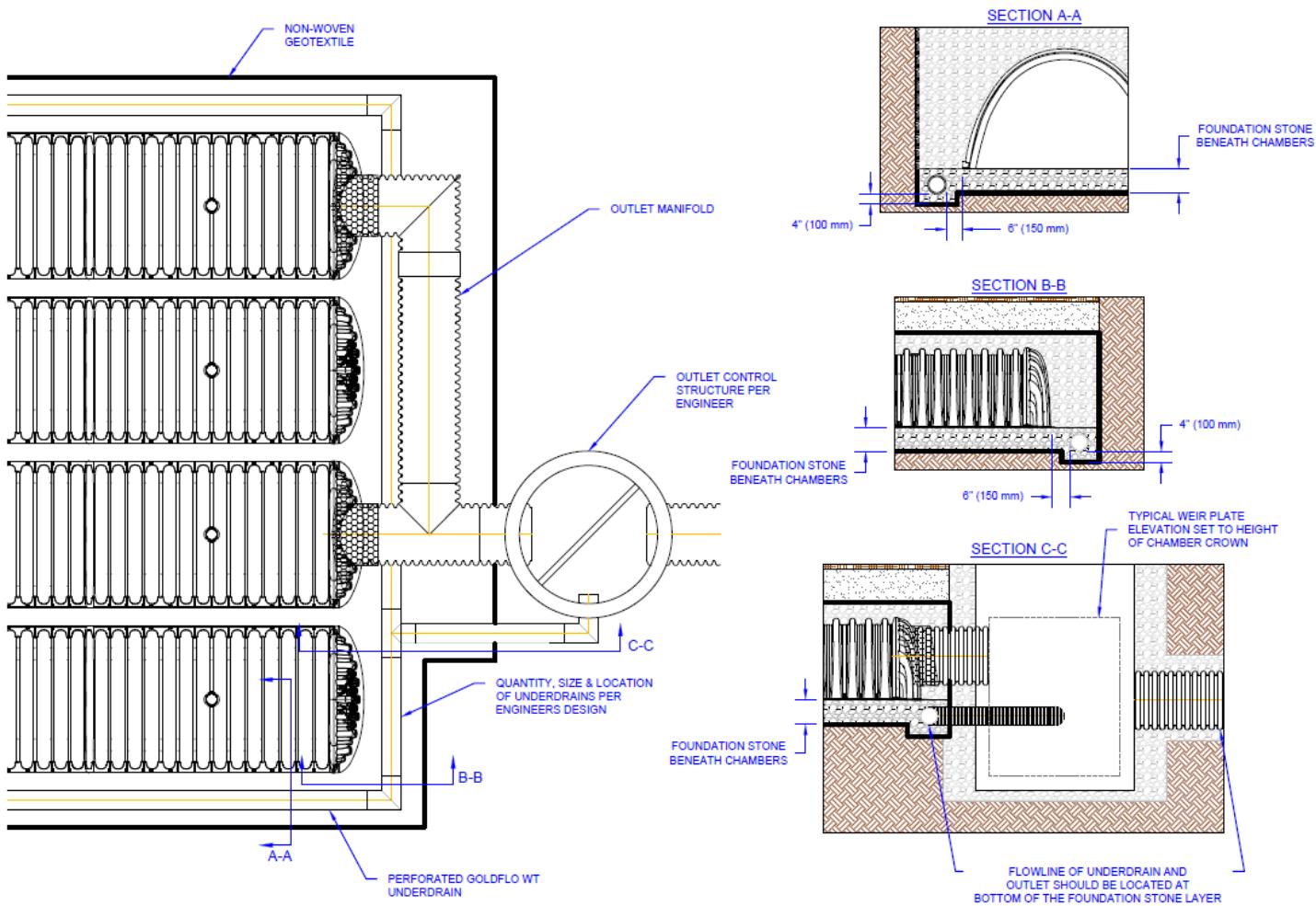


Figure 14 – Optional Underdrain Outlet

One of the benefits of an open bottom chamber system is its ability to infiltrate stormwater back into the ground, providing a situation closer to the pre-developed conditions. A combined outlet approach can help minimize the storage volume needed by providing the ability to infiltrate a portion of the stormwater runoff, while releasing excess flow during large storm events through the drainage system. This can be accomplished by providing an outlet structure or pipe with a higher invert elevation within the HydroStor system. The excess volume is released through an outlet structure while the remaining storm water runoff infiltrates into the ground.

Outlet pipes or structures should be placed along the perimeter of the HydroStor system as shown in Figure 15. An outlet pipe manifold should be connected to the downstream end cap(s) at the designed invert. The sediment row should not be connected to an outlet pipe as that will allow sediment to escape from the system. If one outlet is insufficient for the system, multiple outlets may be installed. It is the design engineers' responsibility to properly design a system that meets the hydraulic and hydrologic requirements of the site to be developed, while meeting all applicable laws.

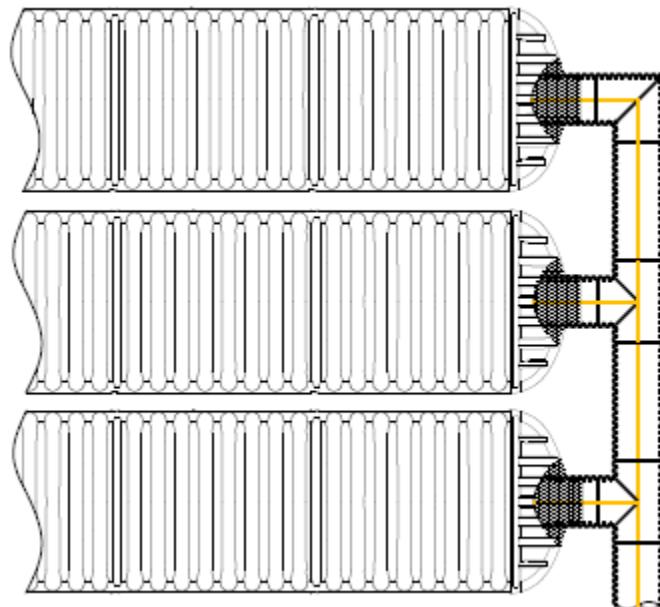


Figure 15 - Outlet Manifold Configuration

9.0 Other Considerations

9.1 Impermeable Liners

In addition to functioning as an open bottom detention system or infiltrating retention system (into the underlying soil), the HydroStor chamber system can be designed as a watertight system, preventing stormwater from infiltrating into the subsurface. A watertight option may be desirable in conditions where the groundwater is contaminated, or underlying conditions could be impacted by excess water infiltration. When it is not desirable for the water to infiltrate into the underlying soil, a thermoplastic liner may be used to provide an impermeable barrier and prevent infiltration into the subsurface. Thermoplastic liner options include:

- Polyvinyl Chloride (PVC), 30 Mil PVC
- Linear Low-Density Polyethylene (LLDPE), 30 Mil LLDPE
- Reinforced Polypropylene (RPP), EPDM, and XR-5

Liners installed with HydroStor chamber system require proper installation in accordance with the manufacturer's installation instructions. It is recommended that an AASHTO M288 Class 2 or 3 non-woven geotextile be placed on both sides of the thermoplastic liner to protect the liner from puncture or protrusions. Refer to Figure 16 below or contact your local Prinsco representative for more information regarding the design and installation of liner systems for storm water applications with HydroStor chamber systems.

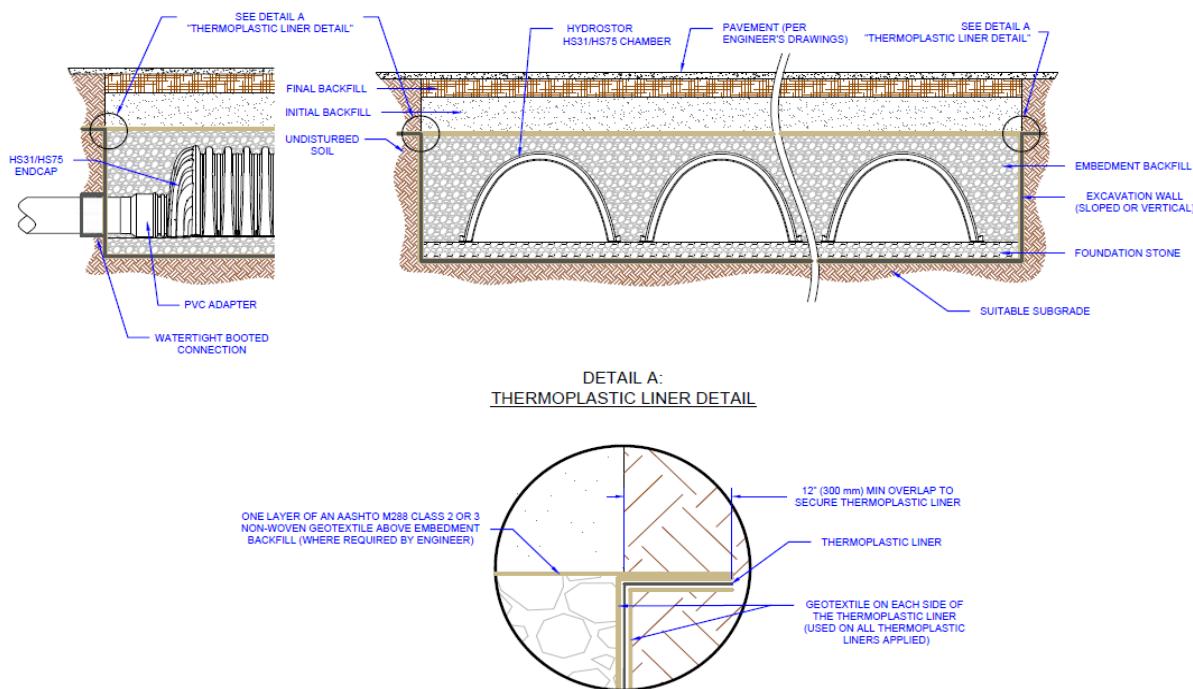


Figure 16 - Impermeable Liner Cross Section

10.0 System Sizing

To assist in the design of the HydroStor chamber systems, a Chamber System Design Aid (XLS) calculator is available on Prinsco's website, www.prinsco.com. This calculator enables the user to input the design criteria and estimates the quantities needed for the system. Contact your local Prinsco representative for further information on use of the calculator.

10.1 Sizing a HydroStor Chamber System

The design of a detention or retention system is the responsibility of the design engineer for the site being developed. The following steps provide guidance and information necessary for the design of the HydroStor system. All calculations and design functions must be accomplished and approved by the design engineer. If you need technical assistance with the layout or design of the HydroStor system contact your local Prinsco representative.

STEP 1: Determine the amount of storage required for the system.

The determination for the volume of water to be stored is the sole responsibility of the design engineer based on local codes, regulations, and design practice.

STEP 2: Determine the number of chambers required.

To determine the number of chambers needed, divide the amount of storage required by the volume of the chamber selected. The volume of storage available per chamber is provided in Table 8.

Table 8 - Chamber & Stone Storage Volumes

Unit & Stone Storage Volume, ft³ (m³)

Stone Foundation Depths, in (mm)	HS75 Chamber	HS31 Chamber
Bare Unit	46.6 (1.32)	15.14 (0.43)
6 (150)	74.99 (2.12)	29.56 (0.84)
9 (225)	78.4 (2.22)	31.8 (0.9)
12 (300)	81.8 (2.32)	34 (0.96)
15 (375)	85.2 (2.41)	36.2 (1.03)
18 (450)	88.6 (2.51)	38.4 (1.09)

STEP 3: Determine the required foundation size for the chambers.

To determine the size of the bed, multiply the number of chambers required by the foundation area per chamber.

HS31 Chamber Foundation Area = 22.0 ft² (2.04 m²)

HS75 Chamber Foundation Area = 33.6 ft² (3.12 m²)

In addition to the foundation area calculated, an additional 1 foot (300 mm) should be added around the chamber foundation to provide for working room and backfill of the chambers.

STEP 4: Determine the amount of aggregate required.

To determine the amount of aggregate required, multiply the number of chambers by the selected foundation depth as shown in Table 9 below.

Table 9 - Stone Required per Chamber / End Cap

Stone Foundation Depths, in (mm)	Stone Volume yd³, US tons (m³, metric tons)	
	HS75 Chamber	HS31 Chamber
6 (150)	2.6, 3.51 (1.99, 3.18)	1.33, 1.8 (1.02, 1.63)
9 (230)	2.91, 3.93 (2.23, 3.57)	1.54, 2.08 (1.18, 1.89)
12 (300)	3.22, 4.35 (2.46, 3.95)	1.74, 2.35 (1.33, 2.13)
15 (380)	3.53, 4.77 (2.7, 4.33)	1.94, 2.62 (1.49, 2.38)
18 (460)	3.84, 5.19 (2.94, 4.71)	2.15, 2.9 (1.64, 2.63)

STEP 5:
Calculate the volume of the excavation required.

The volume of excavation required per chamber is shown in Table 10 and 11 below. In addition, 1 foot (300 mm) of excavation is required around the perimeter and must be included.

Table 10 - HS31 Volume of Excavation Required per Chamber

Stone Foundation Depths, in (mm)	Volume of Excavation yd ³ (m ³)
Chamber	
6 (150)	2.57 (1.97)
12 (300)	2.98 (2.28)
18 (460)	3.39 (2.59)

**Assumes 3" (75 mm) of separation between chamber rows, 12" (300 mm) of perimeter in front of end caps and 16" (410 mm) of cover minimum. If the depth of cover exceeds 18" (450 mm), the volume should be increased 0.81cy (0.62 m³) per foot (300 mm) of depth.*

Table 11 – HS75 Volume of Excavation Required per Chamber

Stone Foundation Depths, in (mm)	Volume of Excavation yd ³ (m ³)
Chamber	
6 (150)	5.57 (4.26)
12 (300)	6.19 (4.73)
18 (460)	6.81 (5.21)

**Assumes 6" (150 mm) of separation between chamber rows, 12" (300 mm) of perimeter in front of end caps and 18" (450 mm) of cover minimum. If the depth of cover exceeds 18" (450 mm), the volume should be increased 1.24cy (0.95 m³) per foot (300 mm) of depth.*

STEP 6: Calculate the amount of nonwoven geotextile required.

The bottom, sides, and top of the embedment stone must be covered with an AASHTO M288 Class 2 or 3 non-woven geotextile. Allowances should be made to include the 2 feet (600 mm) overlap required at all geotextile joints.

STEP 7: Determine the number of End Caps needed

Each row of chambers requires 2 end caps. If a row is terminated and restarted, two additional end caps will be required on that row.

11.0 Structural Cross Sections and Specifications

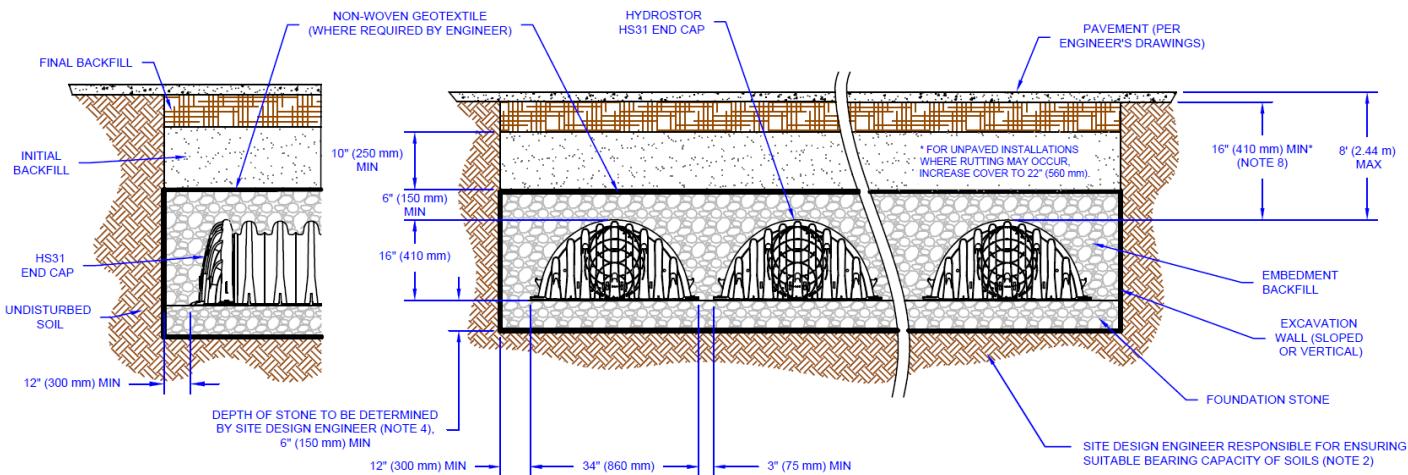


Figure 17 – HS31 Structural Cross Section Detail

HS31 STORMWATER CHAMBER SPECIFICATIONS

1. CHAMBERS SHALL BE HYDROSTOR HS31 OR APPROVED EQUIVALENT.
2. CHAMBERS SHALL BE MADE FROM VIRGIN, IMPACT-MODIFIED POLYPROPYLENE COPOLYMERS.
3. CHAMBER ROWS SHALL PROVIDE CONTINUOUS, UNOBSTRUCTED INTERNAL SPACE WITH NO INTERNAL SUPPORT PANELS THAT WOULD IMPEDE FLOW OR LIMIT ACCESS FOR INSPECTION.
4. THE STRUCTURAL DESIGN OF THE CHAMBERS, THE STRUCTURAL BACKFILL, AND THE INSTALLATION REQUIREMENTS SHALL ENSURE THAT THE LOAD FACTORS SPECIFIED IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SECTION 12.12, ARE MET FOR: 1) LONG-DURATION DEAD LOADS AND 2) SHORT-DURATION LIVE LOADS, BASED ON THE AASHTO DESIGN TRUCK WITH CONSIDERATION FOR IMPACT AND MULTIPLE VEHICLE PRESENCES.
5. CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F2418-12, "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS."
6. ONLY CHAMBERS THAT ARE APPROVED BY THE SITE DESIGN ENGINEER WILL BE ALLOWED. THE CHAMBER MANUFACTURER SHALL SUBMIT THE FOLLOWING UPON REQUEST TO THE SITE DESIGN ENGINEER FOR APPROVAL BEFORE DELIVERING CHAMBERS TO THE PROJECT SITE:
 - A. STRUCTURAL EVALUATION SEALED BY A REGISTERED PROFESSIONAL ENGINEER THAT DEMONSTRATES THAT THE SAFETY FACTORS ARE GREATER THAN OR EQUAL TO 1.95 FOR DEAD LOAD AND 1.75 FOR LIVE LOAD, THE MINIMUM REQUIRED BY ASTM F2787 AND BY AASHTO FOR THERMOPLASTIC PIPE.
 - B. A STRUCTURAL EVALUATION SEAL BY A REGISTERED PROFESSIONAL ENGINEER THAT DEMONSTRATES THAT THE LOAD FACTORS SPECIFIED IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SECTION 12.12, ARE MET. THE 50 YEAR CREEP MODULUS DATA SPECIFIED IN ASTM F2418 MUST BE USED AS PART OF THE AASHTO STRUCTURAL EVALUATION TO CERTIFY LONG-TERM PERFORMANCE.
 - C. STRUCTURAL CROSS SECTION DETAIL ON WHICH THE STRUCTURAL EVALUATION IS BASED.
7. CHAMBERS SHALL BE PRODUCED AT AN ISO 9001 CERTIFIED MANUFACTURING FACILITY.
8. PRIOR TO BEGINNING INSTALLATION OF HYDROSTOR STORMWATER CHAMBERS, A PRECONSTRUCTION MEETING SHALL BE HELD WITH A PRINSCO REPRESENTATIVE AND THE INSTALLERS.
9. HYDROSTOR STORMWATER CHAMBERS SHALL BE INSTALLED IN ACCORDANCE WITH THE PRINSCO "HYDROSTOR CONSTRUCTION GUIDE."
10. HYDROSTOR STORMWATER CHAMBERS SHALL NOT BE INSTALLED ON WET OR UNSTABLE FOUNDATION OR SUBGRADE. FOUNDATION STONE MUST BE LEVEL AND COMPAKTED.
11. PRINSCO RECOMMENDS PRETREATMENT OF STORMWATER RUNOFF USING A PRINSCO STORMWATER QUALITY UNIT AND/OR A SEDIMENT ROW.
12. MAINTAIN MINIMUM SPACING OF 3" BETWEEN CHAMBERS.
13. CONSTRUCTION EQUIPMENT SHALL NOT BE SITUATED ATOP THE CHAMBERS UNTIL SUFFICIENT COVER HAS BEEN ACHIEVED. DUMP TRUCKS, RUBBER TIRE LOADERS, EXCAVATORS, WHEEL OR ROLLER LOADS ARE NOT ALLOWED UNTIL PROPER FILL HEIGHTS HAVE BEEN ACHIEVED. REFER TO PRINSCO "HYDROSTOR CONSTRUCTION GUIDE" FOR SPECIFIC LOADING CRITERIA.
14. EMBEDMENT BACKFILL MUST BE PLACED USING THE FOLLOWING METHODS ONLY:
 - A. BACKFILL WITH AN EXCAVATOR LOCATED OUTSIDE THE EXCAVATION
 - B. BACKFILL WITH A STONE SHOOTER LOCATED OUTSIDE THE EXCAVATION
 - C. BACKFILL AS ROWS ARE BUILT WITH AN EXCAVATOR ON THE SUBGRADE OR FOUNDATION STONE
15. EMBEDMENT BACKFILL SHALL NOT BE PLACED USING THE "DUMP AND PUSH" METHOD. THIS MAY CAUSE DAMAGE TO THE CHAMBERS, WILL RESULT IN IMPROPER INSTALLATION AND WILL VOID THE PRINSCO STANDARD WARRANTY.
16. ONCE SUFFICIENT COVER IS ACHIEVED (16" FOR HS31), GRADING MAY COMMENCE WITH A SMALL DOZER OR SKID LOADER (LESS THAN 4.5 PSI GROUND PRESSURE). EQUIPMENT SHALL ALWAYS TRAVEL PARALLEL TO CHAMBER ROWS. SEE PRINSCO "HYDROSTOR CONSTRUCTION GUIDE" FOR SPECIFIC LOADING CRITERIA.

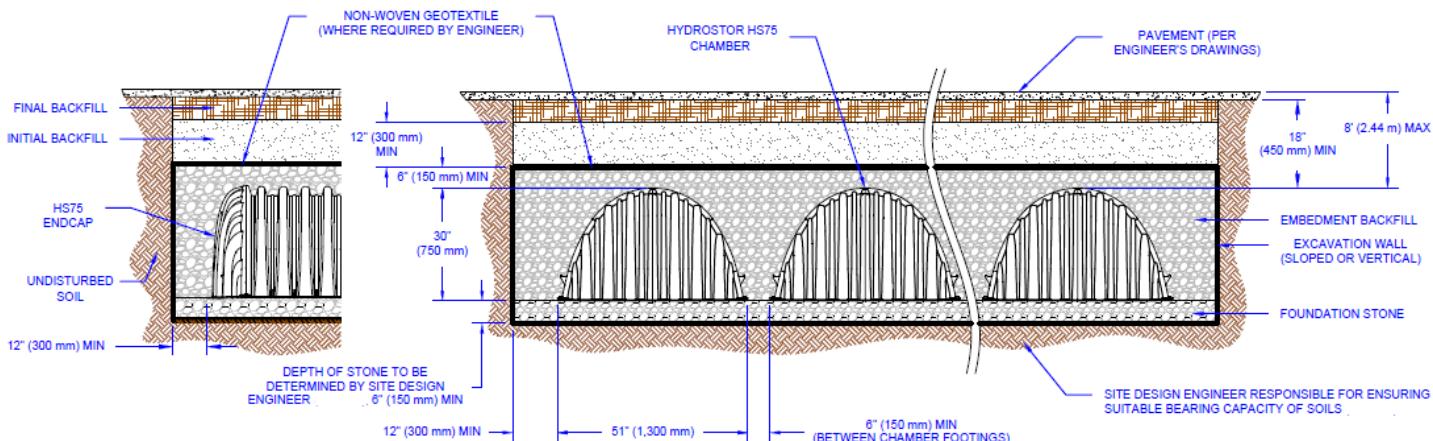


Figure 18 - HS75 Structural Cross Section Detail

HS75 STORMWATER CHAMBER SPECIFICATIONS

1. CHAMBERS SHALL BE HYDROSTOR HS75 OR APPROVED EQUIVALENT.
2. CHAMBERS SHALL BE MADE FROM VIRGIN, IMPACT-MODIFIED POLYPROPYLENE COPOLYMERS.
3. CHAMBER ROWS SHALL PROVIDE CONTINUOUS, UNOBSTRUCTED INTERNAL SPACE WITH NO INTERNAL SUPPORT PANELS THAT WOULD IMPEDE FLOW OR LIMIT ACCESS FOR INSPECTION.
4. THE STRUCTURAL DESIGN OF THE CHAMBERS, THE STRUCTURAL BACKFILL, AND THE INSTALLATION REQUIREMENTS SHALL ENSURE THAT THE LOAD FACTORS SPECIFIED IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SECTION 12.12, ARE MET FOR: 1) LONG-DURATION DEAD LOADS AND 2) SHORT-DURATION LIVE LOADS, BASED ON THE AASHTO DESIGN TRUCK WITH CONSIDERATION FOR IMPACT AND MULTIPLE VEHICLE PRESENCES.
5. CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F2418, "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS."
6. ONLY CHAMBERS THAT ARE APPROVED BY THE SITE DESIGN ENGINEER WILL BE ALLOWED. THE CHAMBER MANUFACTURER SHALL SUBMIT THE FOLLOWING UPON REQUEST TO THE SITE DESIGN ENGINEER FOR APPROVAL BEFORE DELIVERING CHAMBERS TO THE PROJECT SITE:
 - A. A STRUCTURAL EVALUATION SEALED BY A REGISTERED PROFESSIONAL ENGINEER THAT DEMONSTRATES THAT THE SAFETY FACTORS ARE GREATER THAN OR EQUAL TO 1.95 FOR DEAD LOAD AND 1.75 FOR LIVE LOAD, THE MINIMUM REQUIRED BY ASTM F2787 AND BY AASHTO FOR THERMOPLASTIC PIPE.
 - B. A STRUCTURAL EVALUATION SEAL BY A REGISTERED PROFESSIONAL ENGINEER THAT DEMONSTRATES THAT THE LOAD FACTORS SPECIFIED IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SECTION 12.12, ARE MET. THE 50 YEAR CREEP MODULUS DATA SPECIFIED IN ASTM F2418 MUST BE USED AS PART OF THE AASHTO STRUCTURAL EVALUATION TO CERTIFY LONG-TERM PERFORMANCE.
 - C. STRUCTURAL CROSS SECTION DETAIL ON WHICH THE STRUCTURAL EVALUATION IS BASED.
7. CHAMBERS AND END CAPS SHALL BE PRODUCED AT AN ISO 9001 CERTIFIED MANUFACTURING FACILITY.
8. PRIOR TO BEGINNING INSTALLATION OF HYDROSTOR STORMWATER CHAMBERS, A PRECONSTRUCTION MEETING SHALL BE HELD WITH A PRINSCO REPRESENTATIVE AND THE INSTALLERS.
9. HYDROSTOR STORMWATER CHAMBERS SHALL BE INSTALLED IN ACCORDANCE WITH THE PRINSCO "HYDROSTOR CONSTRUCTION GUIDE."
10. HYDROSTOR STORMWATER CHAMBERS SHALL NOT BE INSTALLED ON WET OR UNSTABLE FOUNDATION OR SUBGRADE. FOUNDATION STONE MUST BE LEVEL AND COMPAKTED.
11. PRINSCO RECOMMENDS PRETREATMENT OF STORMWATER RUNOFF USING A PRINSCO STORMWATER QUALITY UNIT AND/OR A SEDIMENT ROW.
12. MAINTAIN MINIMUM SPACING BETWEEN CHAMBERS; 6" FOR HS75
13. CONSTRUCTION EQUIPMENT SHALL NOT BE SITUATED ATOP THE CHAMBERS UNTIL SUFFICIENT COVER HAS BEEN ACHIEVED. DUMP TRUCKS, RUBBER TIRE LOADERS, EXCAVATORS, WHEEL OR ROLLER LOADS ARE NOT ALLOWED UNTIL PROPER FILL HEIGHTS HAVE BEEN ACHIEVED. REFER TO PRINSCO "HYDROSTOR CONSTRUCTION GUIDE" FOR SPECIFIC LOADING CRITERIA.
14. EMBEDMENT BACKFILL MUST BE PLACED USING THE FOLLOWING METHODS ONLY:
 - A. BACKFILL WITH AN EXCAVATOR LOCATED OUTSIDE THE EXCAVATION
 - B. BACKFILL WITH A STONE SHOOTER LOCATED OUTSIDE THE EXCAVATION
 - C. BACKFILL AS ROWS ARE BUILT WITH AN EXCAVATOR ON THE SUBGRADE OR FOUNDATION STONE
15. EMBEDMENT BACKFILL SHALL NOT BE PLACED USING THE "DUMP AND PUSH" METHOD. THIS MAY CAUSE DAMAGE TO THE CHAMBERS, WILL RESULT IN IMPROPER INSTALLATION AND WILL VOID THE PRINSCO STANDARD WARRANTY.
16. ONCE SUFFICIENT COVER IS ACHIEVED (6" FOR HS75), GRADING MAY COMMENCE WITH A SMALL DOZER OR SKID LOADER (LESS THAN 4.5 PSI GROUND PRESSURE). EQUIPMENT SHALL ALWAYS TRAVEL PARALLEL TO CHAMBER ROWS. SEE PRINSCO "HYDROSTOR CONSTRUCTION GUIDE" FOR SPECIFIC LOADING CRITERIA.

12.0 Sediment Row Inspection & Maintenance

12.1 Sediment Row Inspection

Inspecting and maintaining the HydroStor chamber system is critical to ensuring the stormwater system will perform as designed. Buildup of debris in the chamber system may result in ineffective operation or complete failure of the system. Inspection of chamber sediment row can be done through the manhole or optional inspection ports on the chamber sediment row. All local and OSHA rules must be followed for confined space entry.

Inspection ports offer the convenience of inspection from the ground surface without concern of confined space entry. (Figure 19) Inspection ports allow for visual inspection with the use of a flashlight or a long stick to measure the level of sediment.

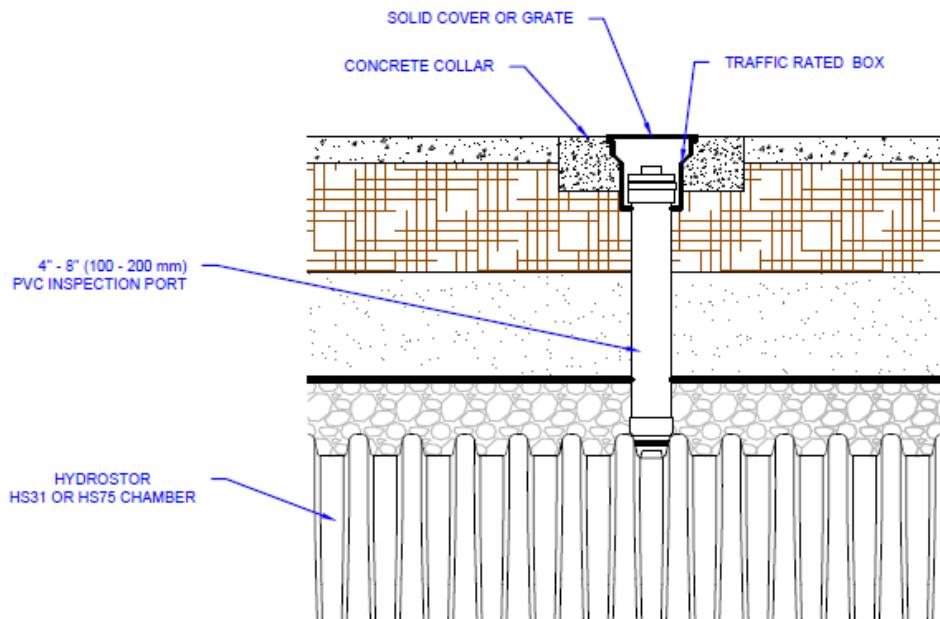


Figure 19 - Inspection Ports for Chamber Sediment Rows

12.2 Inspection Frequency

Initial inspection of the system is recommended soon after the site construction is completed. Every effort should be made to keep sediment and debris out of the stormwater system during construction; however, this is typically the time when excess amounts of sediment builds up. It is recommended that the system be inspected and cleaned, if necessary, before passing the responsibility over to the site owner.

Inspection frequency will vary based on the system design and requirements. A system inspection schedule should be developed for each individual system, with the industry standard being a minimum of twice per year. It is recommended that the inspections be tracked on an Inspection and Maintenance log sheet.

12.3 Sediment Row Maintenance

Maintenance of the chamber sediment row is recommended after an average depth of 3" (75 mm) of sediment has been collected. The most common method for cleaning is done by using a high pressure water jet and a vacuum truck (JetVac system). The sediment should be directed to the inlet control structure sump and vacuumed out. The JetVac process shall only be performed on HydroStor chamber rows that have AASHTO M288 Class 1 woven geotextile over the foundation stone.

13.0 General Notes

1. The installation of HydroStor chamber systems must be done in accordance with the latest published HydroStor Installation/Construction Guidelines. Visit Prinsco's website, www.prinsco.com, for the most up to date installation guidelines.
2. Prinsco offers installation consultations to installing contractors. Contact your local Prinsco representative a minimum of 30 days prior to system installation to arrange a pre-installation consultation. Prinsco representatives are able to answer questions or address comments about the HydroStor chamber systems and explain the minimum installation requirements prior to the system being installed. Call 1-800-992-1725 to speak to a local Prinsco representative.
3. The minimum cover height is 16" (410 mm) and 18" (450 mm) for HydroStor HS31 and HS75 chambers, resp. The minimum cover height is measured from the top of the chambers to the bottom of the pavement (asphalt, concrete, etc.) For installations without pavement, where rutting from vehicles may occur, the minimum cover height should be increased to 22" (560 mm) and 24" (600 mm) for the HS31 and HS75 chambers, resp.
4. The maximum cover height for HS31 and HS75 chambers is 8' (2.44 m) measured from the top of the chambers to the top of the pavement.
5. It is the contractor's responsibility to report any discrepancies with the bearing capacity of the subgrade materials to the design engineer.
6. AASHTO M288 Class 2 or Class 3 non-woven geotextile must be used for the separation layer as indicated on the project plans.
7. Stone placement between chamber rows and around the perimeter of the chambers must follow the instructions indicated in the most recent HydroStor Installation/Construction Guidelines.
8. Backfilling over the chambers must follow the requirements indicated in the most recent HydroStor Installation/Construction Guidelines.
9. The contractor must refer to the most recent HydroStor Installation/Construction guidelines for a Table of Acceptable Vehicle Loads at various depths of cover. It is the contractor's responsibility to ensure that vehicles exceeding those listed in the table do not travel over the chambers. The use of temporary fencing, warning tape, and/or signs are commonly used methods to keep oversized vehicles out of the construction zone.
10. The contractor must apply erosion and sediment control measures to protect the chamber system during all phases of site construction based on the local codes and design engineer's requirements.
11. HydroStor chambers are covered under Prinsco's Limited Warranty – Terms & Conditions. Contact your local Prinsco Representative for more information.



When it comes to stormwater chambers, the road from good to great leads directly to HydroStor®, the industry's highest performing chambers. They meet or exceed proven ASTM structural, design and product standards, while including features that matter to the designer and installer.

HS31

6" (150 mm)	minimum foundation	6" (150 mm)
3" (75 mm)	chamber spacing	6" (150 mm)
34" (864 mm)	chamber width	51" (1,295 mm)
12" (300 mm)	backfill at edge of system	12" (300 mm)
16" (406 mm)	chamber height	29.7" (754 mm)
16" (410 mm)	minimum cover	18" (450 mm)
8' (2.4 m)	maximum burial depth	8' (2.4 m)

HS75

Available Sizes



HS31

29.6 ft ³ (.84 m ³)/chamber	Installed Storage Capacity*	75 ft ³ (2.12 m ³)/chamber
16" (406 mm)	Height	29.7" (754 mm)
34" (864 mm)	Width	51" (1,295 mm)
87.3" (2,217 mm)	Length	87.1" (2,212 mm)
Lightweight Option	Special Features	Integrated Handles
Meets or Exceeds	ASTM Standards	Meets or Exceeds



HS75

The HydroStor **HS31** stores 4.15 ft³ of stormwater per linear foot (0.39 m³ per linear meter) or 29.6 ft³ (.84 m³) per chamber, and is designed for projects with limited burial depths

The HydroStor **HS75** stores 10.6 ft³ of stormwater per linear foot (0.98 m³ per linear meter) or 75 ft³ (2.12 m³) per chamber, and is designed for projects with limited burial depths.

Prinsco's HydroStor chambers meet rigorous ASTM standards and include value-added design features that take this new chamber option from good to great.

Performance

- High performance polypropylene material
- Meets or exceeds **ASTM F2418** product standard & **ASTM F2787** design standard
- Meets **AASHTO H20** live load & **HL93** design load requirements
- Advanced injection molding technology for maximum structural performance

Mobile Resources

More about HydroStor®

- Installation Videos
- Installation Documents
- Specifications
- Technical Notes

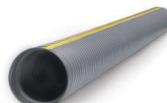


Prinsco products are fully supported by our engineering team and are designed, manufactured and tested to meet/exceed the high performance needs of the construction market. Prinsco's engineering, quality control and production teams are committed to a continuous process of innovation and product development. We are focused on current and future market needs centered around environmental sustainability, water quality, stormwater management and performance advancement.

COLLECTION & CONVEYANCE



GOLDFLO®



GOLDPRO STORM®

TREATMENT



STORMWATER QUALITY

STORAGE



HYDROSTOR®



UNDERGROUND DETENTION